# **GUJARAT TECHNOLOGICAL UNIVERSITY** BE - SEMESTER-VI • EXAMINATION – SUMMER 2016

#### Subject Code: 2163509 Date: 13/05/2016 Subject Name: Liquid Effluent Treatment - II Time: 10:30 am – 01:00 am **Total Marks: 70 Instructions:** 1. Attempt all questions. 2. Make suitable assumptions wherever necessary. 3. Figures to the right indicate full marks. Q.1 (a) Design a Tube Settler Module of a Square Cross section with the following data: 07 1) Avg output required from settle = 6 MLD2) Loss of water in desludging = 2% of output 3) Diameter of tube = 0.05 m4) Angle of inclination = 605) Kinematic viscosity = $1.01 \times 10^{-6} \text{ m}^2/\text{s}$ 6) Critical settling velocity = 120 m/day7) Length of tube = 1 m8) C.S Sq area of tube = $0.05m \times 0.05m$ (b) Design a clarifier with a flow of 2 MLD with following data. 07 1) HRT = 3 hrs. 2) Height of clarifier = 63) C-C distance between notches = 20 cm

Q.2 (a) 1) Trickling filter has a diameter of 20 m and a liquid depth of 2.5 m. Calculate the **04** organic loading rate for an influent of 10.0 MLD having 220 mg/l of BOD.

2) The cumulative flow of wastewater reaching at the end of time period to a treatment03plant in a day varies as shown below. Determine the capacity of an equalization tankfor the given flow variation.

ſ	Time (h)	0	2	4	6	8	10	12	14	16	18	20	22	24
Γ	Cumulative	0	25	50	75	100	120	130	140	150	160	170	198	225
	Flow $(m^3)$													

- (b) Design two identical bar rack screens for a treatment plant handling the peak flow of 07 210 MLD. Assume the other conditions as follows:
  - 1) Velocity through screen at peak flow = 1.2 m/s
  - 2) Clear spacing between the bars = 25 mm
  - 3) Bar size = 10 mm x 50 mm
  - 4) Angle of bars with inclination = 80
  - 5) Peak factor = 2
  - 6) Depth of flow in chamber = 1.1 m
  - 7) K = 1.43
  - 8)  $G = 9.8 \text{ m/s}^2$

### OR

(b) Assuming suitable design criteria, design an aerated grit chamber for an average 07 municipal wastewater flow of 10 MLD. Take peaking factor as 2.5 and 5 minutes as a detention time.

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- 1) Liquid Depth = 2.0 m
- 2) Width-Depth ratio = 1.5:1
- 3) Air supply rate =  $0.3 \text{ m}^3/\text{min-m}$
- 4) Grit settling rate =  $0.015 \text{ m}^3/1000 \text{ m}^3$  of flow
- Q.3 (a) Design an oil and grease trap to remove 160 mg/l of oil and grease from a flow of 07 50000 m<sup>3</sup>/d of domestic wastewater.
  - (b) Find the terminal settling velocity of the spherical particle with diameter 0.05 mm of **07** specific gravity 2.65 settling through water having kinematic viscosity 1.004 x  $10^{-6}$  m<sup>2</sup>/s.

## OR

- Q.3 (a) Enlist the different types of methods used to remove Amonical Nitrogen from the **07** wastewater stream. Explain any one in detail.
  - (b) For an activated sludge process system with recycle, calculate the MCRT for the given 07 data.
    - 1) Daily average wastewater flow = 10 MLD
    - 2) Effluent flow rate of wastewater = 9.92 MLD
    - 3) Sludge wasting flow = 0.08 MLD
    - 4) HRT = 6 hrs
    - 5) Active biomass conc in the reactor = 3000 mg/l
    - 6) Biomass conc in the effluent = 20 mg/l
    - 7) Biomass conc in the return line = 9000 mg/l.
- Q.4 (a) If BOD<sub>5</sub> of a sample measured at 20 °C is 250 mg/l and the reaction constant k is 0.35 **07**  $d^{-1}$ , calculate the ultimate and the 3 day BOD at 20 °C.
  - (b) If 1 MLD flow of domestic wastewater with settled BOD<sub>5</sub> of 250 mg/l is treated in the **07** convectional activated sludge plant reactor at 0.3 F/M ratio to obtain 85% of BOD removal efficiency, estimate the net surplus sludge produced per day. Assume Y = 0.6 &  $K_d = 0.06 d^{-1}$

### OR

- Q.4 (a) Assuming food to microbes ratio equal to 0.25 and hydraulic residence time of 6 hours, 07 compute the value of MLVSS to be maintained in the reactor of a convectional activated sludge plant designed to treat 5 MLD settled wastewater with 200 mg/l of BOD<sub>5</sub>.
  - (b) 1) Assuming the diameter of the clarifier to be 20 m and the wastewater flow rate of 03 10 MLD. Calculate the detention time and surface loading rate of the clarifier having a wastewater depth of 2.5 m.

2) For a circular diameter of 20 m diameter, determine the weir loading rate for a **03** wastewater flow rate of 10 MLD.

3) A floating sticks travels a distance of 15 m in 30 seconds in a reactor tank having 2 **01** m width and 1.5 m depth. Determine the flow and flow through velocity.

- Q.5 (a) Enlist the different types of methods used to remove Biological Phosphorus from the **07** wastewater stream. Explain any one in detail.
  - (b) Define the following: 1) Surface overflow rate 2) Weir overflow rate 3) Detention 07 time 4) Flow through velocity 5) Volumetric Loading rate 6) Endogenous decay constant 7) MCRT

- Q.5 (a) Design a Primary settling tank to treat domestic wastewater flow of a town having **07** 500000 population.
  - 1) Average rate of water supply = 250 lpcd
  - 2) Wastewater generation rate = 80% of water supplied
  - 3) Suspended solids in water = 250 mg/l
  - 4) BOD<sub>5</sub> of wastewater = 200 mg/l
  - 5) SLR = 40 m<sup>3</sup>/m<sup>2</sup>\*day (at average flow), 100 m<sup>3</sup>/m\*day (at peak flow)
  - 6) Detention time = 2 hr
  - 7) Solids concentrations of sludge = 6 %
  - (b) Raw sewage with BOD of 220 mg/l is applied to a convectional sewage treatment plant 07 based on activated sludge process. If the primary treatment units remove 25% of BOD, determine the MLVSS to be maintained in the reactor to control an F/M ratio of 0.6, assuming a wastewater flow rate of 0.5 MLD.

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