Seat No.: \_\_\_\_\_

Enrolment No.\_\_\_\_\_

## **GUJARAT TECHNOLOGICAL UNIVERSITY**

ME – SEMESTER I (NEW) – • EXAMINATION – SUMMER 2016

Subject Code: 2712009

# Subject Name: Advanced Foundation Engineering

Date:19/05/2016

Time:02:30 pm to 05:00 pm

**Total Marks: 70** 

Instructions:

- 1. Attempt all questions.
- 2. Make suitable assumptions wherever necessary.
- **3.** Figures to the right indicate full marks.
- 4. Use of IS: 8009(part I II), IS:2911-2010, IS:5249-1992, IS:1904, IS:6403, IS:2950(part I) and other related IS code are permitted.
- Q.1 (a) Explain the vertical and horizontal load resisting mechanism in shallow and 07 deep foundation (pile foundation) along with suitable sketches.
  - (b) Enlist all possible degree of freedoms (vibration mode) to be considered for analysis of machine foundation along with sketch. Also draw a diagram of spring-mass model for a practically possible combined mode of vibration with respective stiffness.
- Q.2 (a) The successive peaks from a free vibration record obtained from initial 07 displacement(sudden release) test on a block resting on a soil are tabulated are:

Time (s)	0	0.02	0.04	0.06	0.08	0.10	0.12	0.14
Peak amplitude (mm)	16	-12	8	-6	4	-3	2	-1.5

Time (s)	0.16	0.18	0.20	0.22	0.24	0.26	0.28	0.30
Peak amplitude	1	-0.75	0.5	-0.38	0.25	-0.19	0.13	-0.10
(mm)								

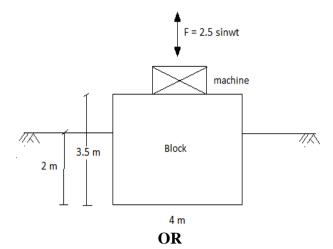
Compute the natural frequency of vibration of the block and also the damping in the foundation as per IS: 5249-1992.

(b) A reciprocating machine is symmetrically mounted on a block of size 4 m x 3m x 3.5 m high as shown in figure below. The soil at the site is sandy in nature having  $\emptyset = 35^{\circ}$  and  $\gamma_{sat} = 20$  kN/m<sup>3</sup>. The water table lies at a depth of 3 m below the ground surface. The block is embedded in the ground by 2 m depth. The machine vibrating vertically at a speed of 250 rpm generates maximum unbalance vertical force 2.5 kN.

The machine weight is small in comparison to the weight of the foundation. The limiting amplitude of the machine is 150 microns. A block resonance test was conducted at the site to evaluate the dynamic elastic constants. The data obtain from the test are:

$$\label{eq:cu} \begin{array}{l} Cu = 3.62 \ x \ 10^4 \ kN/m^3. \\ G = 1.10 \ x \ 10^4 \ kN/m^2. \\ E = 2.98 \ x \ 10^4 \ kN/m^2. \\ \mu = 0.35. \end{array}$$

Determine the natural frequency and amplitudes by weightless spring-mass model.



- (b) Determine the natural frequency and amplitudes in above examples (Q-2, b) by 07 Elastic half space approach.
- Q.3 (a) Design a friction pile group to carry a load of 2500 kN including the weight of the pile cap at a site where the soil is uniform clay to a depth of 18 m underlain by rock. The average unconfined compressive strength of the clay is 70 kN/m<sup>2</sup>. A factor of safety 2.5 is required against shear failure. Assume adhesion factor α = 1. The pile is of 10 m length with 0.5 m diameter.
  - (b) In the above example (Q-3, a), assuming the clay to be of normal sensitivity and normally loaded with liquid limit of 60 %, compute the settlement of pile group as per guidelines given in IS: 8009(part-II). The unit weight of clay  $\gamma = 16$  kN/m<sup>3</sup>.

#### OR

- **Q.3** (a) A precast circular concrete pile of 450 mm diameter in a prebored hole is of 15 m length installed in a deposit of sand. If the co-efficient of subgrade reaction  $\Pi$ h = 10.5 x 10<sup>6</sup> N/m<sup>3</sup>. Find the deflection of the pile head as per IS:2911:2010 considering it as free head pile under a horizontal force of 35 kN. Assume  $E_{pile} = 20 \text{ kN/mm}^2$ .
  - (b) Explain Pile Load test.
- **Q.4** (a) A rectangular shallow footing has a size of 1.8m x 3m has to be transmit a load of a column at a depth of 1.5m. Calculate the safe bearing capacity of a footing as per IS: 6403 with factor of safety 3 against shear failure. The soil has following properties:  $C = 8 \text{ kN/m}^2$ ,  $\emptyset = 32.5^0$ ,  $\gamma = 18 \text{ kN/m}^3$ . The depth of footing is 1.5 m from ground level.
  - (b) Carry out the size proportioning of a TRAPEZOIDAL combined footing for a 07 given data:
    - 1. LHS column ( $W_1 = 2000 \text{ kN}$ ) and RHS column ( $W_2 = 1500 \text{ kN}$ )
    - 2. Centre to centre distance between column axis = 7 m.
    - 3. Property lines: 1 m on left side from axis of LHS column.
    - 4. SBC = 140 kPa.
    - Also draw only S.F. diagram mentioning typical values.

#### OR

- Q.4 (a) A footing 4m x 2m in plan, transmits a pressure of 150 kN/m2 on a cohesive soil having  $E_s = 6 \times 10^4 \text{ kN/m}^2$  and  $\mu = 0.5$ . Determine the immediate settlement of the footing at the centre as per IS:8009(part- I), assuming it to be
  - 1-) a flexible footing and
  - 2-) a rigid footing.
  - (b) Answer the following w.r.t. Raft foundation:
    - 1-) What is minimum depth of foundation.
    - 2-) Under which situations one can go for raft foundation.
    - 3-) State the criteria for considering raft as rigid or flexible for analysis.
    - 4-) Draw a typical sketch for various types of raft foundation.

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- Q.5 (a) State the difference between Elastic half space approach and Winkler springmass model approach for carrying out Soil-Structure Interaction analysis.
  - (b) Suggest different measures to be taken for foundation on expansive soil along 07 with suitable justification.

### OR

- Q.5 (a) Enlist the conditions responsible for Liquefaction to take place in soil. Also 07 suggest the different way to mitigate the Liquefaction with suitable justification.
  - (b) Explain the stability analysis of a Well foundation.

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